



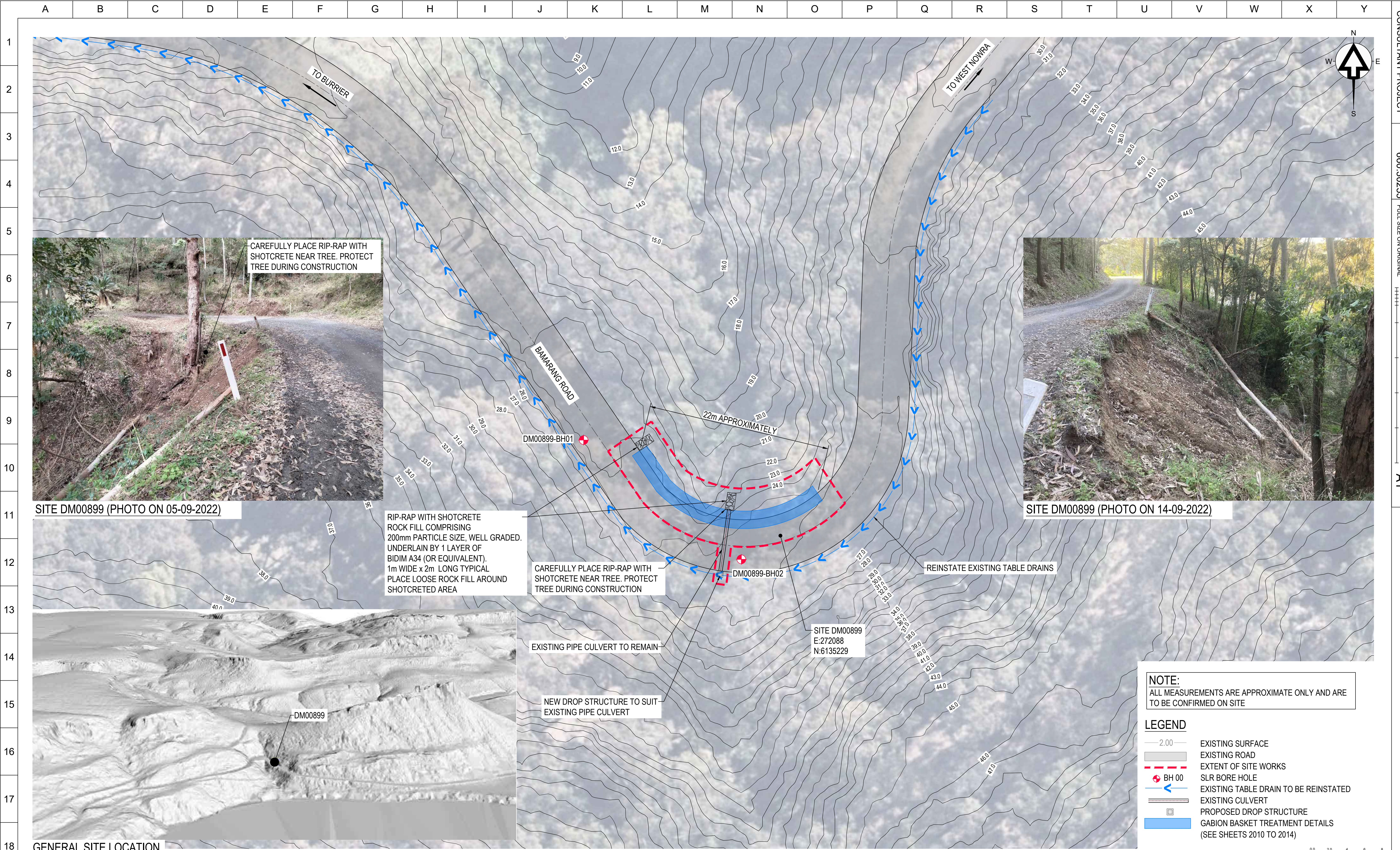



		A	B	C	D	E	F	G	H	I	J	K	L	M	N	O	P	Q	R	S	T	U	V	W	X	Y	CONSULTANT PROJECT 660.30255 FULL SIZE ON ORIGINAL 0 10 20 40 60 80 100 A1																							
1		DRAWING SCHEDULE (1 OF 2)												DRAWING SCHEDULE (2 OF 2)												COORDINATES TABLE																								
2		DRAWING NUMBER				DRAWING TITLE								DRAWING NUMBER				DRAWING TITLE								SITE NUMBER				SITE LOCATION				EASTING				NORTHING												
		660.30255-G-1000				COVER SHEET & SITES LOCALITY PLAN								660.30255-G-1200				SITE DM00583 - GENERAL ARRANGEMENT LAYOUT PLAN								DM00865				Abernathys Rd				266450				6145038												
		660.30255-G-1001				DRAWING SCHEDULE & COORDINATES TABLE								660.30255-G-1201				SITE DM00583 - CROSS SECTIONS SHEET								SH00290				Bamarang Rd				271979				6135292												
3		660.30255-G-1002				GENERAL NOTES SHEET 1																				DM00706				Browns Mountain Rd				273244				6145779												
		660.30255-G-1003				GENERAL NOTES SHEET 2																				AC00088				Bunkers Hill Rd				274285				6159006												
		660.30255-G-1004				GENERAL NOTES SHEET 3																				DM00548				Bunkers Hill Rd				275018				6160966												
4						660.30255-G-1010								SITE DM00865 - GENERAL ARRANGEMENT LAYOUT PLAN								660.30255-G-1220				SITE NH00011 - GENERAL ARRANGEMENT LAYOUT PLAN								DM00549				Bunkers Hill Rd				274459				6159432				
																														DM00869				Bunkers Hill Rd				274354				6159116								
		660.30255-G-1020				SITE SH00290 - GENERAL ARRANGEMENT LAYOUT PLAN								660.30255-G-1230				SITE DM00513 - GENERAL ARRANGEMENT LAYOUT PLAN								DM00870				Bunkers Hill Rd				274402				6159173												
5																										SH00282				Burrier Rd				267939				6137163												
		660.30255-G-1030				SITE DM00706 - GENERAL ARRANGEMENT LAYOUT PLAN								660.30255-G-1231				SITE DM00513 - CROSS SECTIONS SHEET								SH00288				Burrier Rd				271051				6135673												
		660.30255-G-1031				SITE DM00706 - CROSS SECTIONS SHEET								660.30255-G-1232				SITE DM00513 - DETAILS SHEET 1								SH00292				Burrier Rd				268817				6136420												
6																										DM00555				Foremans Rd				287060				6155133												
		660.30255-G-1040				SITE AC00088 - GENERAL ARRANGEMENT LAYOUT PLAN								660.30255-G-1240				SITE DM00863 - GENERAL ARRANGEMENT LAYOUT PLAN								DM00882				Foremans Rd				287028				6155134												
																										DM00610				Hughes Rd				267764				6140045												
7		660.30255-G-1050				SITE DM00548 - GENERAL ARRANGEMENT LAYOUT PLAN								660.30255-G-1250				SITE SH00276 - GENERAL ARRANGEMENT LAYOUT PLAN								DM00611				Hughes Rd				267775				6140040												
		660.30255-G-1051				SITE DM00548 - DETAILS SHEET																				DM00805				Kangaroo Valley Rd				279846				6152399												
													660.30255-G-1260				SITE DM00808 - GENERAL ARRANGEMENT LAYOUT PLAN								DM00806				Kangaroo Valley Rd				279546				6152162													
8	660.30255-G-1060				SITE DM00549 - GENERAL ARRANGEMENT LAYOUT PLAN								660.30255-G-1261				SITE DM00808 - CROSS SECTIONS SHEET								DM00533				Mount Scanzi Rd				268294				6149554													
													660.30255-G-1262				SITE DM00808 - TREATMENT ELEVATION SHEET								DM00577				Mount Scanzi Rd				269153				6150610													
																									DM00718				Mount Scanzi Rd				269141				6150636													
9	660.30255-G-1070				SITE DM00869 - GENERAL ARRANGEMENT LAYOUT PLAN								660.30255-G-1270				SITE MT00008 - GENERAL ARRANGEMENT LAYOUT PLAN								DM00755				Mount Scanzi Rd				268187				6149366													
	660.30255-G-1071				SITE DM00869 - CROSS SECTIONS SHEET								660.30255-G-1271				SITE MT00008 - CROSS SECTIONS SHEET								DM00757				Mount Scanzi Rd				268466				6149581													
													660.30255-G-1272				SITE MT00008 - ELEVATION & DETAILS SHEET								DM00864				Mount Scanzi Rd				268507				6149629													
10	660.30255-G-1080				SITE DM00870 - GENERAL ARRANGEMENT LAYOUT PLAN								660.30255-G-1280				SITENR00016 - GENERAL ARRANGEMENT LAYOUT PLAN								NH00013				Mount Scanzi Rd				268449				6149585													
													660.30255-G-1281				SITE NR00016 - CROSS SECTIONS SHEET								DM00583				Upper Kangarooo River Rd				278778				6156987													
	660.30255-G-1100				SITE SH00288 - GENERAL ARRANGEMENT LAYOUT PLAN																				DM00828				Upper Kangarooo River Rd				280258				6161915													
11													660.30255-G-1290				SITE AQ00001 - GENERAL ARRANGEMENT LAYOUT PLAN								NH00011				Upper Kangarooo River Rd				279513				6157304													
	660.30255-G-1110				SITE SH00292 - GENERAL ARRANGEMENT LAYOUT PLAN								660.30255-G-1300				SITE DM00523 - GENERAL ARRANGEMENT LAYOUT PLAN								DM00513				Wattamolla Rd				288246				6154987													
																									DM00863				Wattamolla Rd				285173				6154673													
12	660.30255-G-1120				SITES DM00882 & DM00555 - GENERAL ARRANGEMENT LAYOUT PLAN								660.30255-G-1310				SITE DM00822 - GENERAL ARRANGEMENT LAYOUT PLAN								SH00276				Wogamia Rd				272677				6137591													
	660.30255-G-1121				SITE DM0555 - CROSS SECTIONS SHEET																				DM00808				Woodhill Mountain Rd				288281				6154144													
	660.30255-G-1122				SITE DM00555 - DETAILS SHEET								660.30255-G-1320				SITE DM00890 - GENERAL ARRANGEMENT LAYOUT PLAN								MT00008				Woodhill Mountain Rd				288085				6154219													
13	660.30255-G-1123				SITE DM00882 - CROSS SECTIONS SHEET																				NR00016				Yalwal Rd				271466				6134664													
													660.30255-G-1330				SITE DM00899 - GENERAL ARRANGEMENT LAYOUT PLAN								AQ00001				Upper Kangarooo River Rd				278908				6157117													
	660.30255-G-1130				SITES DM00610 & DM00611 - GENERAL ARRANGEMENT LAYOUT PLAN																				DM00523				Suffolk Road				277515				6116275													
14	660.30255-G-1140				SITE DM00805 - GENERAL ARRANGEMENT LAYOUT PLAN								660.30255-G-2000				TYPICAL DETAILS & SPECIFICATIONS SHEET 1								DM00822				Mount Scanzi Rd				269174				6150402													
													660.30255-G-2001				TYPICAL DETAILS & SPECIFICATIONS SHEET 2								DM00890				Burrier Rd				269100				6136495													
	660.30255-G-1150				SITE DM00806 - GENERAL ARRANGEMENT LAYOUT PLAN								660.30255-G-2002				TYPICAL DETAILS & SPECIFICATIONS SHEET 3								DM00899				Bamarang Rd				272066				6135240													
15	660.30255-G-1151				SITE DM00806 - DETAILS SHEET								660.30255-G-2003				TYPICAL DETAILS & SPECIFICATIONS SHEET 4																																	
													660.30255-G-2010				TYPICAL DETAILS & SPECIFICATIONS SHEET 5																																	
	660.30255-G-1160				SITE DM00533 - GENERAL ARRANGEMENT LAYOUT PLAN								660.30255-G-2011				TYPICAL DETAILS & SPECIFICATIONS SHEET 6																																	
16	660.30255-G-1170				SITES DM00577 & DM00718 - GENERAL ARRANGEMENT LAYOUT PLAN								660.30255-G-2012				TYPICAL DETAILS & SPECIFICATIONS SHEET 7																																	
													660.30255-G-2013				TYPICAL DETAILS & SPECIFICATIONS SHEET 8																																	
	660.30255-G-1180				SITE DM00755 - GENERAL ARRANGEMENT LAYOUT PLAN								660.30255-G-2014				TYPICAL DETAILS & SPECIFICATIONS SHEET 9																																	
17																																																		
	660.30255-G-1190				SITES DM00757, NH00013 & DM00864 - GENERAL ARRANGEMENT LAYOUT PLAN																																													
18																																																		
PLOT DATE 22-Feb-2023 4:11:29 PM	REVISIONS							THIS DRAWING IS THE PROPERTY OF SLR CONSULTING AUSTRALIA AND MUST NOT BE RETAINED, COPIED OR USED WITHOUT THE CONSENT OF THE COMPANY.				DRAWN: WP/GL	DATE: 19.10.2022	 <div>LEVEL 1, THE CENTRAL BUILDING INNOVATION CAMPUS, SQUIRES WAY NORTH WOLLONGONG NSW 2500 AUSTRALIA T: +61 2 4249 1004 F: +61 2 4249 1000 www.slrconsulting.com</div>	 <div>FOR CONSTRUCTION</div>	CLIENT: SHOALHAVEN CITY COUNCIL																																		
																PROJECT: LANDSLIDE REMEDIATION DESIGN OF VARIOUS SITES																																		
										THIS DRAWING IS NOT TO BE USED FOR CONSTRUCTION UNLESS ENDORSED BELOW						DESIGN: MB/DT	DATE: 20.10.2022	DRAWING TITLE: DRAWING SCHEDULE & COORDINATES TABLE																																
																DRG. CHECK: MB	DATE: 24.10.2022																																	
		1	24.02.2023	ISSUED FOR CONSTRUCTION						Responsible Principal Signature	Date	DES. CHECK: DT	DATE: 25.10.2022			The content contained within this document may be based on third party data. SLR Consulting Australia Pty Ltd does not guarantee the accuracy of any such information.								A1	DO NOT SCALE THIS DRAWING IF IN DOUBT ASK	SCALE: N/A	DRAWING NUMBER: 660.30255-G-1001				ISSUE 1																			

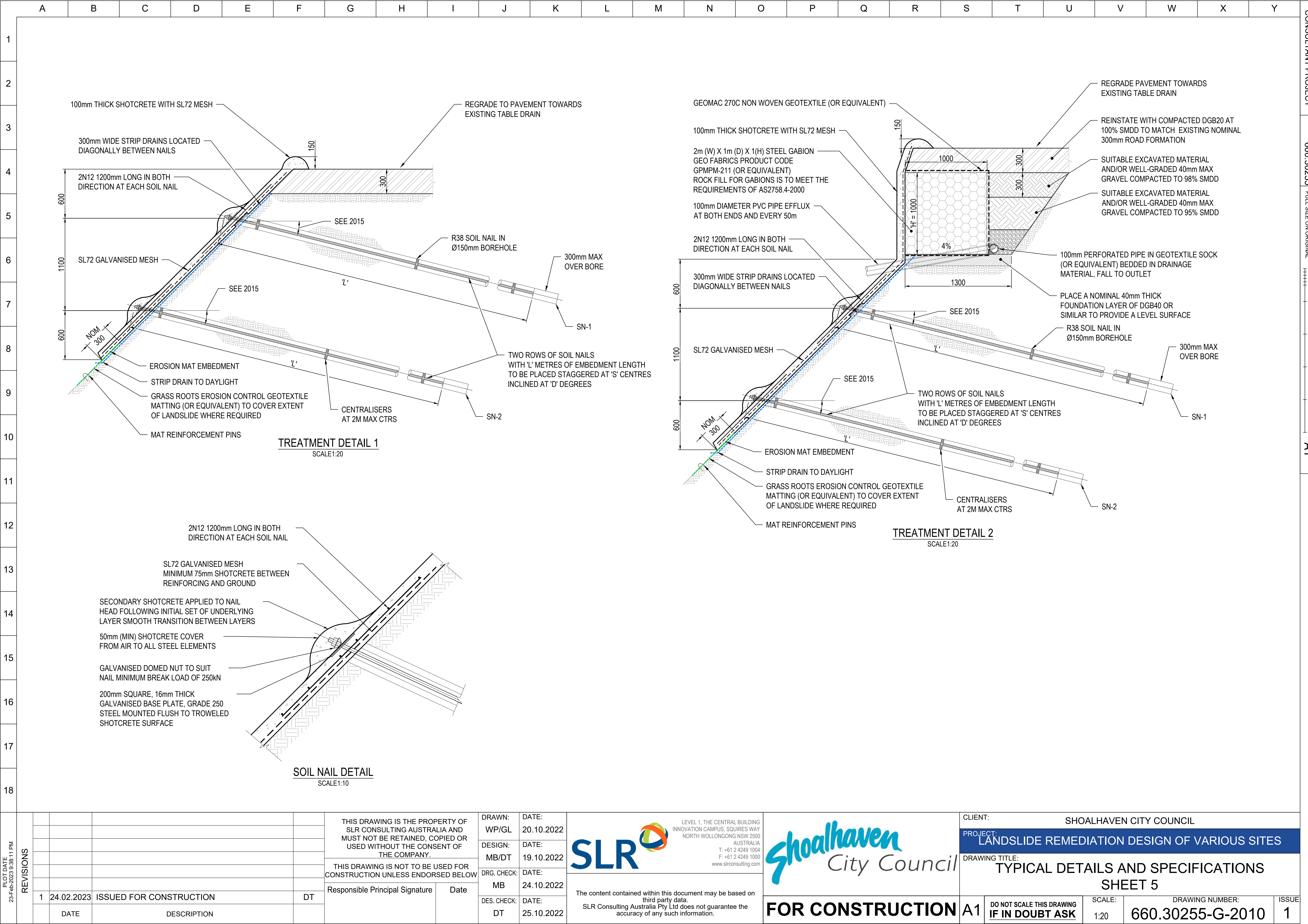
		A	B	C	D	E	F	G	H	I	J	K	L	M	N	O	P	Q	R	S	T	U	V	W	X	Y							
1	2	DEFINITIONS										24. OBTAIN NECESSARY PERMITS AND APPROVALS TO WORK ON SITE. NOTIFY RELEVANT SERVICE AUTHORITIES BEFORE COMMENCING WORK ON SITE.										53. GENERAL WASTE (RUBBISH) IS NOT TO BE ALLOWED TO LIE OR ACCUMULATE ON THE SITE. KEEP ALL DOCKETS/RECEIPTS FOR WATE MANAGEMENT/DISPOSAL AND FORWARD COPIES TO THE PRINCIPAL'S REPRESENTATIVE.											
		1. PRINCIPAL - THE PRINCIPAL IS IDENTIFIED AS SHOALHAVEN CITY COUNCIL.										25. SERVICES ARE TO BE LOCATED ON SITE PRIOR TO CONSTRUCTION COMMENCING.										54. ONLY VIRGIN EXCAVATED NATURAL MATERIAL (VENM) CAN BE IMPORTED ON SITE UNLESS APPROPRIATE TESTING AND DOCUMENTATION IS PROVIDED.											
		2. GEOTECHNICAL DESIGN REPRESENTATIVE - SUITABLY QUALIFIED GEOTECHNICAL ENGINEER APPOINTED BY THE PRINCIPAL TO PROVIDE ON-SITE VERIFICATION OF THE CONSTRUCTION.										26. DURING CONSTRUCTION STRUCTURES SHALL BE MAINTAINED IN A STABLE CONDITION AND NO PART SHALL BE OVERLOADED. TEMPORARY SUPPORTS SHALL BE PROVIDED BY THE BUILDER IN ORDER TO KEEP THE WORKS AND ANY EXCAVATIONS STABLE AT ALL TIMES.										55. KEEP ALL DOCUMENTS/RECORDS OF THE TRANSPORT AND USE OF ATERIAL IMPORTED ONTO SITE AND FORWARD COPIES TO THE PRINCIPAL'S REPRESENTATIVE.											
		3. CONTRACTOR - THE ENTITY UNDERTAKING THE CONSTRUCTION.										27. PROVIDE SAFETY BARRIERS AT EDGES OF OPENINGS AND ELEVATED AREAS. ENSURE WORKERS ARE APPROPRIATELY RESTRAINED/ TETHERED/ HARNESSD DURING WORKS WHERE THE LIKELIHOOD AND CONSEQUENCE OF A FALL IS REASONABLY SIGNIFICANT.										TRAFFIC CONTROL											
		4. UNO - UNLESS NOTED OTHERWISE.										28. REVIEW ADEQUACY OF WORKING SPACE AVAILABLE FOR CONSTRUCTION ACTIVITIES. ENSURE SEPARATION OF PLANT AND PERSONNEL ON SITE, INCLUDING MOVEMENTS OF BOTH.										56. ESTABLISH TRAFFIC CONTROL (IF REQUIRED) AT PROJECT BOUNDARIES IN ACCORDANCE WITH AN APPROPRIATE TRAFFIC CONTROL PLAN APPROVED BY THE PRINCIPAL.											
		GENERAL NOTES										29. LOCATE LIFTING SLEW AND LAY DOWN AREAS AWAY FROM REGULAR CONSTRUCTION TRAFFIC.										57. CONSTRUCTION WARNING SIGNS AND TRAFFIC SPEED SIGNS TO BE ESTABLISHED IN GENERAL ACCORDANCE WITH TRAFFIC CONTROL PLAN APPROVED BY THE PRINCIPAL.											
		5. THE GEOLOGICAL PROFILE AND GEOTECHNICAL CONDITIONS SHOWN ON THE DRAWINGS ARE INDICATIVE AND HAVE BEEN INFERRED FROM LIMITED INVESTIGATIONS. SHOULD THE ENCOUNTERED GEOLOGICAL PROFILE DIFFER FROM THAT SHOWN, THE DESIGN SHALL BE REVIEWED TO ENSURE SUITABILITY OF THE DESIGN TO ENCOUNTERED CONDITIONS.										30. WRITTEN RISK ASSESSMENTS ARE ADVISED FOR ACCESS TO OPEN EXCAVATIONS.										58. A WASTE CLASSIFICATION HAS NOT BEEN CARRIED OUT FOR THIS SITE. HOWEVER, IF POTENTIAL CONTAMINANTS ARE DISCOVERED DURING WORKS A WASTE CLASSIFICATION MUST BE CARRIED OUT BY THE CONTRACTOR, IN ACCORDANCE WITH REGULATORY REQUIREMENTS.											
		6. PRIOR TO CONSTRUCTION SITE SET-UP, THE CONTRACTOR SHALL SUBMIT THE FOLLOWING TO THE PRINCIPAL FOR REVIEW.										31. PROVIDE ACCESS AND EGRESS TO EXCAVATIONS APPROPRIATE IN CASE OF INUNDATION, COLLAPSE OR ENGULFMENT.										EARTHWORKS											
		A. ALL SAFE WORK METHOD STATEMENTS AND PLANS										32. SEEK ADVICE FROM SUITABLY QUALIFIED GEOTECHNICAL OR STRUCTURAL ENGINEER PRIOR TO OPERATION OF HEAVY SURFACE PLANT AND EQUIPMENT OR STOCKPILING MATERIAL NEAR OPEN EXCAVATIONS OR EXISTING RETAINING STRUCTURES.										59. STRIP OFF ALL VEGETATION, RUBBISH AND TOPSOIL CONTAINING ORGANIC OR ROOT MATTER FROM THE AREA OF THE WORKS AND REMOVE FROM SITE/ STOCKPILE FOR RE-USE.											
		B. CONSTRUCTION STAGING PLANS										33. SEEK ADVICE FROM SUITABLY QUALIFIED GEOTECHNICAL FOR ALL TEMPORARY BATTERS.										60. WHERE REQUIRED, EXCAVATE SITE TO THE EXTENT DETAILED IN THE DETAILED SECTION DRAWING.											
		C. CONSTRUCTION PROGRAM										34. SEEK ADVICE FROM SUITABLY QUALIFIED STRUCTURAL ENGINEER BEFORE CORING, CHASING, CUTTING OR REMOVAL OF EXISTING CONCRETE AND REINFORCEMENT.										61. FOR EACH SITE, THE FOUNDATION LEVEL WILL VARY DEPENDING ON GROUND CONDITIONS ENCOUNTERED ON THE SITE AND SHALL BE CONFIRMED ON SITE BY A SUITABLY QUALIFIED GEOTECHNICAL ENGINEER.											
		D. CONSTRUCTION ENVIRONMENTAL MANAGEMENT PLAN (CEMP)										35. MAKE WORK AREAS SAFE WHERE STRUCTURAL ELEMENTS ARE DAMAGED, CRACKED OR HAVE SUFFERED SIGNIFICANT SECTION LOSS BEFORE ALLOWING GENERAL CONSTRUCTION OR REPAIR ACCESS.										62. PRIOR TO ANY FILLING, THE EXPOSED SUBGRADE SHALL BE PROOF ROLLED WITH A 12 TONNE STATIC MASS SMOOTH DRUM ROLLER IN THE PRESENCE OF THE PRINCIPAL'S REPRESENTATIVE OR GEOTECHNICAL ENGINEER AND ANY SOFT OR YIELDING MATERIALS REMOVED AND REPLACED WITH APPROVED FILLING COMPACTED AS HEREFTER SPECIFIED.											
		E. PROJECT QUALITY PLAN										36. REPORT SIGNIFICANT SECTION LOSS OR CORROSION FLAKING BEFORE STARTING PAINTING OR REPAIRS. CONSULT SUITABLY QUALIFIED STRUCTURAL ENGINEER IF SECTION LOSS OR EXTENSIVE CORROSION FLAKING PRESENT BEFORE PROCEEDING WITH WORK.										63. IMPORTING FILL MATERIAL TO SITE: SEE NOTE NO. 54.											
		APPROVAL OF THE DOCUMENTATION BY THE PRINCIPAL SHALL CONSTITUTE A HOLD POINT.										37. UNTIL PERMANENT SUPPORT IS PROVIDED, PROVIDE TEMPORARY SUPPORT FOR SECTIONS OF EXISTING STRUCTURES WHICH ARE TO BE ALTERED AND WHICH NORMALLY RELY FOR SUPPORT ON WORK TO BE DEMOLISHED.										64. FILL SHALL BE SOUND WELL GRADED MATERIAL WITH A HIGH GRANULAR CONTENT AND SHALL BE THE BEST OF EXCAVATED MATERIALS FROM THE SITE, OR APPROVED SOUND IMPORTED MATERIAL FREE OF RUBBISH, PLASTIC CLAY OR LARGE PIECES THAT WOULD PRECLUDE COMPACTION.											
		7. SITE TO BE RETURNED TO ORIGINAL CONDITION OR BETTER AT THE COMPLETION OF THE WORKS										38. PROVIDE SUPPORT TO ADJACENT STRUCTURES WHERE NECESSARY, SUFFICIENT TO PREVENT DAMAGE RESULTING FROM THE WORKS.										65. FILL SHALL BE SPREAD IN LAYERS NOT EXCEEDING 150mm AND COMPACTED AT OPTIMUM MOISTURE CONTENT ±2% TO NOT LESS THAN THE REQUIRED MAXIMUM DRY DENSITY.											
		8. NOMINATION OF PROPRIETARY ITEMS DOES NOT INDICATE EXCLUSIVE PREFERENCE BUT INDICATES THE REQUIRED PROPERTIES OF THE ITEM. SIMILAR ALTERNATIVES HAVING THE REQUIRED PROPERTIES MAY BE OFFERED FOR APPROVAL BY THE DESIGN REPRESENTATIVE.										39. LATERAL SUPPORTS: PROVIDE LATERAL SUPPORT AT LEAST EQUAL TO THAT GIVEN BY THE STRUCTURE TO BE DEMOLISHED, USING SHORING.										66. DENSITY TESTING OF FILLING (AND BASE COURSE WHERE APPLICABLE) SHALL BE CARRIED OUT AT THE RATE OF 1 TEST PER 200 SQUARE METRES EACH LAYER (MINIMUM 2 TESTS).											
		9. DIMENSIONS ARE IN MILLIMETRES, LEVELS ARE IN METRES UNO, CHAINAGES ARE IN METRES UNO. REDUCED LEVELS RELATE TO AUSTRALIAN HEIGHT DATUM.										40. VERTICAL SUPPORTS: PROVIDE SUPPORT WHERE NECESSARY USING PILING OR UNDERPINNING, OR BOTH.										67. WHERE REQUIRED, THE FOUNDATION LAYER SHALL BE, DRY, SMOOTH, LEVEL AND GRADED TO THE PROFILE AS SHOWN IN THE DETAILED DRAWING. IT SHALL BE FREE OF SURFACE IRREGULARITIES, LOOSE OR UNSTUITABLE MATERIAL.											
		2	3	10. ALL DIMENSIONS AND SETTING OUT SHALL BE VERIFIED ON SITE BY THE CONTRACTOR BEFORE COMMENCING WORK. HAVE SURVEY AND SETTING OUT UNDERTAKEN BY A REGISTERED SURVEYOR. ALL DIMENSIONS DO NOT OBTAIN DIMENSIONS BY SCALING OF THE DRAWINGS.										ENVIRONMENTAL SITE MANAGEMENT										68. WHERE REQUIRED, A BLINDNG LAYER TO INFILL VOIDS, UNEVEN GROUND AND CAVITIES AT FOUNDATION LEVEL TO BE CREATED WITH SITE WON GRANULAR SOILS AND ROCKFILL TO CREATE A SMOOTH, EVEN, STABLE WORKING PLATFORM. GRANULAR BLINDING MATERIAL SHALL COMPRISE OF GRANULAR, SITE WON SOIL FREE OF UNSTUITABLE MATERIAL AND SHALL BE COMPACTED USING MANUAL TECHNIQUES (E.G., HAND TAMPING, WALK-BEHIND COMPACTOR) TO MINIMUM RELATIVE DENSITY OF 70%.									
11. VERIFY ON SITE SETTING OUT DIMENSIONS AND EXISTING MEMBER SIZES SHOWN ON DRAWINGS BEFORE SHOP DRAWINGS, CONSTRUCTION AND FABRICATION IS COMMENCED.										41. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE THAT ALL MEASURES ARE TAKEN DURING THE COURSE OF CONSTRUCTION TO PREVENT SEDIMENT EROSION AND POLLUTION OF THE DOWNSTREAM SYSTEM.										69. THE CONTRACTOR SHALL PROGRAM AND UNDERTAKE THE EARTHWORKS OPERATIONS SUCH THAT WORKING AREAS ARE ADEQUATELY DRAINED DURING CONSTRUCTION. THE SURFACE SHALL BE GRADED AND SEALED OFF TO REMOVE DEPRESSIONS WHICH WOULD ALLOW WATER TO POND AND PENETRATE THE UNDERLYING MATERIAL. ANY DAMAGE RESULTING FROM FAILURE TO COMPLY WITH THESE REQUIREMENTS SHALL BE RECTIFIED AT THE CONTRACTOR'S EXPENSE.													
12. REFER DISCREPANCIES TO THE PRINCIPAL BEFORE PROCEEDING WITH WORK.										42. TEMPORARY EROSION AND SEDIMENT CONTROL (ESC) MEASURES SHALL BE PROVIDED IN ACCORDANCE WITH INTERNATIONAL EROSION CONTROL ASSOCIATION (IECA) BEST PRACTICE EROSION AND SEDIMENT CONTROL GUIDELINES (2008) AND BE MAINTAINED IN OPERATIVE CONDITION AT ALL TIMES AND SHALL REMAIN IN PLACE FOR THE DURATION OF THE WORK.										70. HAVE TESTING PERFORMED BY AN INDEPENDENT NATA (NATIONAL ASSOCIATION OF TESTING AUTHORITIES) ACCREDITED AUTHORITY AND PROVIDE TEST REPORTS TO THE PRINCIPAL. THE CONTRACTOR SHALL BEAR ALL COSTS ASSOCIATED WITH COMPLIANCE TESTING.													
13. WORKMANSHIP AND MATERIALS TO COMPLY WITH REQUIREMENTS OF AUSTRALIAN STANDARDS, NATIONAL CONSTRUCTION CODE (NCC) AND BY-LAWS AND ORDINANCES OF RELEVANT BUILDING AUTHORITIES. ALL STANDARDS REFERRED TO ARE THOSE CURRENT (AS AMENDED) AT COMMENCEMENT OF CONTRACT.										43. SEDIMENT AND EROSION CONTROLS MUST BE IN PLACE PRIOR TO THE COMMENCEMENT OF ANY EARTHWORKS OR DEMOLITION ACTIVITY. THE LOCATION OF SUCH DEVICES IS GENERALLY AT THE END OF THE DIRECTION OF FLOW BEFORE REACHING THE DOWNSTEAM SYSTEM. FINAL POSITION SHALL BE DETERMINED ON SITE.																							
14. WHERE NEW WORK ABUTS EXISTING, PROVIDE SMOOTH TRANSITION FREE OF ABRUPT CHANGES.										44. INSTALL TEMPORARY SEDIMENT BARRIERS TO ALL INLET PITS LIKELY TO COLLECT SILT LADEN WATER UNTIL SURROUNDING AREAS ARE PAVED OR REVEGETATED.																							
15. PROTECT EXISTING STRUCTURES FROM DAMAGE OR CRACKING. MAKE GOOD ANY DAMAGE TO EXISTING ELEMENTS AT COMPLETION OF WORKS.										45. ALL SILT FENCES AND BARRIERS ARE TO BE MAINTAINED IN GOOD ORDER AND REGULARLY DESILTED DURING THE CONSTRUCTION PERIOD.																							
16. IMPLEMENT SOIL AND WATER MANAGEMENT PROCEDURES TO AVOID EROSION, CONTAMINATION AND SEDIMENTATION OF SITE, SURROUNDING AREAS AND DRAINAGE SYSTEMS.										46. STOCKPILES OF LOOSE MATERIALS SUCH AS SAND, SOIL AND GRAVEL MUST BE COVERED WITH GEOTEXTILE. PLASTIC SHEETING OR MEMBRANE MUST NOT BE USED. SAFETY BARRICADING SHALL BE USED TO ISOLATE STOCKPILES OF SOLID MATERIALS SUCH AS STEEL REINFORCING. SEDIMENT FENCE IS TO BE PLACED DOWNSLOPE OF ALL STOCKPILES.																							
17. DISPOSE OF SURPLUS MATERIAL OFF SITE IN ACCORDANCE WITH LOCAL AUTHORITY WASTE REGULATIONS.										47. ANY WASTE GENERATED, INCLUDING EXCAVATED MATERIALS, SHALL BE REMOVED FROM THE SITE AND DISPOSED OF APPROPRIATELY, ACCORDING TO THE WASTE CLASSIFICATION. GENERAL WASTE (RUBBISH) IS NOT TO BE ALLOWED TO LIE OR ACCUMULATE ON THE SITE. KEEP ALL DOCKETS/RECEIPTS FOR WASTE MANAGEMENT/DISPOSAL AND FORWARD COPIES TO THE PRINCIPAL'S REPRESENTATIVE.																							
18. TEMPORARY WORKS AND CONSTRUCTION METHODS ARE THE RESPONSIBILITY OF THE CONTRACTOR.										48. ALL VEHICLES LEAVING THE SITE MUST HAVE CLAY AND SOIL SHAKEN OFF AS PRACTICALLY AS POSSIBLE.																							
19. GIVE TWO WORKING DAYS (48 HOURS) NOTICE SO THAT INSPECTION OF CRITICAL STAGES OF WORK (INCLUDING HOLD POINTS AND WITNESS POINTS) MAY BE MADE.										49. TRUCKS REMOVING EXCAVATED/DEMOLISHED MATERIAL SHALL TRAVEL ON STABILISED CONSTRUCTION PATHS. MATERIAL ARE TO BE TAKEN TO THE TRUCKS TO REDUCE TRUCK MOVEMENTS ON SITE. TRUCKS TO BE LIMITED TO SINGLE UNIT HEAVY RIGID VEHICLES (NO SEMI TRAILERS).																							
20. INSPECTIONS AND REVIEWS UNDERTAKEN BY THE PRINCIPAL OR OTHERS DO NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR COMPLIANCE WITH THE DRAWINGS OR SPECIFICATIONS.										50. DURING TRENCH EXCAVATION ALL SPOIL SHALL BE MOUNDED ON THE UPHILL SIDE OF TRENCHES AND PLACEMENT IS TO COMPLY WITH THE PRINCIPAL'S REQUIREMENT.																							
21. AT THE COMPLETION OF THE WORKS, THE CONTRACTOR SHALL PREPARE AND SUBMIT A 'HANDOVER' PACKAGE TO THE PRINCIPAL FOR APPROVAL. THE PACKAGE SHALL INCLUDE AT A MINIMUM:										WASTE AND CONTAMINATION																							
A. DRILLING RECORDS																																	
B. RFCS, NCR'S, CERTIFICATES AND OTHER FORMS AS NOMINATED BY THE PRINCIPAL.																																	
C. APPROVAL OF THE HANDOVER PACKAGE BY THE PRINCIPAL SHALL CONSTITUTE A HOLD POINT.																																	
3	4			SAFETY IN DESIGN																													
				22. THE SAFETY RISK MITIGATION ITEMS BELOW ARE BASED ON SLR'S DESIGN EXPERIENCE AND DO NOT NECESSARILY ACCOUNT FOR ALL CONSTRUCTION, OPERATION, MAINTENANCE AND DEMOLITION SAFETY RISKS. BASED ON INFORMATION AVAILABLE WHEN THIS DRAWING WAS MADE, IN ITS CAPACITY AS DESIGNER ONLY, SLR HAS TRIED TO IDENTIFY SAFETY RISKS PERTAINING TO CONSTRUCTION, OPERATION, MAINTENANCE AND DEMOLITION PHASES OF THE ASSET. INCLUSION (OR NOT) OF ANY ITEM DOES NOT REDUCE OR LIMIT OBLIGATIONS OF CONSTRUCTOR, USER, MAINTAINER AND DEMOLISHER TO UNDERTAKE APPROPRIATE RISK MANAGEMENT ACTIVITIES TO REDUCE RISK AND IS NOT AN ADMISSION BY SLR THAT INCLUSION OF ANY ITEM IS DESIGNER'S RESPONSIBILITY.																													
				23. CARRY OUT WORK IN A SAFE MANNER IN ACCORDANCE WITH APPLICABLE LEGISLATION, STATUTORY REGULATIONS, BY-LAWS OR RULES. CONTRACTOR IS RESPONSIBLE FOR OCCUPATIONAL HEALTH AND SAFETY OF SITE PERSONNEL AND GENERAL PUBLIC IN ACCORDANCE WITH ALL CURRENT WORK HEALTH AND SAFETY ACTS, LEGISLATIVE REQUIREMENTS, ASSOCIATED REGULATIONS AND CODES OF PRACTICE, INDUSTRIAL AGREEMENTS AND ACCEPTED INDUSTRY PRACTICE.																													
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		EXCAVATION																																																									
		71. SUB VERTICAL EXCAVATION MUST NOT EXCEED 1.5m DEPTH PRIOR TO INSPECTION BY A SUITABLY QUALIFIED GEOTECHNICAL ENGINEER.																										99. BOTTOM COVER FOR FOOTINGS SHALL BE 75mm UNLESS NOTED OTHERWISE.																STRUCTURAL STEEL															
		72. WHERE UNFAVOURABLE EXCAVATION CONDITIONS EXIST SUCH AS SHALLOW LARGE FLOATING BOULDERS, SEEK ENGINEERING ADVICE FROM A SUITABLY QUALIFIED GEOTECHNICAL ENGINEER.																										100. DETAILS OF THE PROPOSED MIX TO BE SUBMITTED & APPROVAL OBTAINED PRIOR TO POURING ANY CONCRETE.																124. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS4100 AND AS1554 EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.															
		73. WHERE SHALLOW BEDROCK IS ENCOUNTERED PRIOR TO DESIGN FOUNDATION DEPTH, SEEK ENGINEERING ADVICE FROM A SUITABLY QUALIFIED GEOTECHNICAL ENGINEER.																										101. IN LIEU OF TRIAL MIX OF CONCRETE TO BE USED, RECENT TRIALS AND PRODUCTION RUNS FOR SIMILAR MIX DESIGNS MAY BE SUBMITTED FOR CONSIDERATION AS EVIDENCE OF COMPLIANCE WITH THE REQUIREMENTS OF THIS SPECIFICATION WITH REGARD TO MIX DESIGN.																125. ALL MATERIAL TO BE GRADE 250 HOT ROLLED PLATES COMPLYING WITH AS 3678 U.N.O.															
		74. TEMPORARY CUT FACES TO BE BATTERED BACK TO A SUITABLE SLOPE TO MAINTAIN STABILITY AT ALL TIMES.																										102. THE CONTRACTOR SHALL ALLOW AT LEAST 1 TEST SECTION EVERY 25 CUBIC METRE OF CONCRETE MIX BROUGHT TO SITE TO DEMONSTRATE COMPLIANCE WITH THIS DOCUMENT. THE FOLLOWING TEST CYLINDERS AND TEST DATA SHALL BE OBTAINED FROM EACH TEST SECTION: A. 7 DAY COMPRESSIVE STRENGTH - 3 CYLINDERS B. 14 DAY COMPRESSIVE STRENGTH - 3 CYLINDERS C. 28 DAY COMPRESSIVE STRENGTH - 3 CYLINDERS																126. WELDING SHALL BE CARRIED OUT IN ACCORDANCE WITH AS 1554.1 AND SHALL BE GP CATEGORY U.N.O.															
		75. DELINEATE A MINIMUM EXCLUSION ZONE FROM THE EDGE OF THE EXCAVATION EQUAL TO THE HEIGHT OF EXCAVATION AT ALL TIMES. NO SURCHARGE LOADS ARE TO THE PLACED WITHIN THIS ZONE SUCH AS VEHICLES AND /OR CONSTRUCTION MATERIALS.																										103. THE AVERAGE STRENGTH OF THE THREE CYLINDERS TESTED AT 28 DAYS SHALL EXCEED THE SPECIFIED CHARACTERISTIC STRENGTH BY AT LEAST 1.65 TIMES THE STANDARD DEVIATION OF THE RESULTS FOR THAT TRIAL MIX.																127. WELDING CONSUMABLES SHALL BE E49XX OR W50X.															
		76. IF ENCOUNTERED, CARE SHALL BE TAKEN TO MINIMISE DISTURBANCE TO THE UNDERLYING DRYSTONE WALL. ANY DISLODGED DRYSTONE AND/OR VOID CREATED BY EXCAVATION SHALL BE REPLACED AND INFILLED WITH GRANULAR FILL.																										104. CONCRETE NOT IN ACCORDANCE WITH THIS SPECIFICATION OR CONCRETE WHICH IS DEFECTIVE SHALL, AT THE DIRECTION OF THE PRINCIPAL, BE REMOVED FROM THE JOB AND REPLACED BY THE CONTRACTOR.																128. INSPECTION SHALL BE CARRIED OUT TO AS 1554.1															
		77. EXCAVATED MATERIALS SHALL BE STOCKPILED WITH HEIGHT NOT EXCEEDING 2 M.																										105. THE CONCRETE SHALL BE PLACED IN SUCH A MANNER TO AVOID SEGREGATION OR LOSS OF MATERIALS. MAXIMUM FALL OF CONCRETE 1500mm OR USE ENCLOSED CHUTES OR SIMILAR.																129. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT STEELWORK IS SECURELY BRACED TEMPORARILY AS NECESSARY TO STABILISE THE STRUCTURE DURING ERECTION.															
		78. CREATE SEPARATE STOCKPILES FOR DIFFERENT SOIL TYPES. DO NOT MIX SUBGRADE WITH PAVEMENT OR TOPSOIL. PROVIDE ADEQUATE WATERING, DRAINAGE AND EROSION CONTROL.																										106. ALL EXTERNAL HORIZONTAL SURFACES TO HAVE A NOMINAL SURFACE FALL TO PREVENT WATER COLLECTION AND PONDING.																130. THE BUILDER SHALL PROVIDE ALL CLEATS AND DRILL ALL HOLES NECESSARY FOR FIXING STEEL TO STEEL WHETHER OR NOT DETAILED ON THE DRAWINGS.															
		79. DO NOT ALLOW TRAFFIC ON STOCKPILES.																										CONCRETE CURING																SEALANTS AND FILLERS															
		80. ALL FINAL SURFACES SHALL BE CONSTRUCTED TO MATCH EXISTING LEVELS UNLESS OTHERWISE APPROVED BY THE PRINCIPAL.																										107. CURING COMPOUNDS SHALL ONLY BE USED ON APPROVAL FROM THE PRINCIPAL. THEY SHALL BE WAX BASED, BE SUITABLY COLOURED FOR IDENTIFICATION PURPOSES, AND SHALL BE TESTED AND MEET ALL OF THE REQUIREMENTS OF AS 3799 - "LIQUID MEMBRANE-FORMING CURING COMPOUNDS FOR CONCRETE".																131. WHERE JOINTS DAYLIGHT AT FORMED JOINTS OR EDGES, THE SEALANT (BOTH TEMPORARY AND PERMANENT) MUST EXTEND DOWN THE VERTICAL FACE OF JOINTS TO PREVENT THE INGRESS OF INCOMPRESSIBLES DURING SUBSEQUENT PAVING. DIMENSIONS MUST BE EQUIVALENT TO THOSE FOR THE TOP JOINT BUT ROTATED 90°.															
		DRAINAGE (GENERAL)																										108. CURING COMPOUNDS SHALL BE SUITABLY COLOURED FOR IDENTIFICATION PURPOSES.																132. SEALANTS AND FILLERS MUST COMPLY WITH RMS R83.															
		81. PRIOR TO CONSTRUCTION OF THE SURFACE DRAIN SPECIAL CONSIDERATION SHALL BE MADE IN CONSTRUCTING THE CUTOFF SUBSOIL DRAINAGE WHICH SHALL BE CONNECTED TO THE LOCAL DRAINAGE NETWORK.																										109. THE CONTRACTOR SHALL DEMONSTRATE TO THE PRINCIPAL THAT THE CURING COMPOUND IS SUITABLE FOR ALL AMBIENT TEMPERATURES, AND THE PARTICULAR CURING COMPOUND SHALL BE APPROVED PRIOR TO USE.																DRILLING OF SOIL NAIL HOLES															
		82. RESHAPE THE EXISTING SURFACE DRAINAGE AND LINED WITH 40MPA SHOTCRETE WITH DOSAGE RATE OF 15KG/M3.																										110. CURING COMPOUNDS SHALL NOT BE BASED ON PVA OR CHLORINATED RUBBER.																133. DRILLING FOR NAILS AND ANCHORS IS EXPECTED TO ENCOUNTER RELATIVELY HIGH STRENGTH COLLUVIUM BOULDERS AND BEDROCK. SELECTION OF DRILLING EQUIPMENT TO BE SUITABLE FOR EXPECTED GROUND CONDITIONS. USE ROTARY OR ROTARY-PERCUSSION METHODS AND EQUIPMENT FOR DRILLING TO ENSURE MINIMAL REMOLDING OF IN-SITU MATERIALS WITHIN THE DRILL HOLES. DO NOT USE DRILLING FLUIDS OTHER THAN AIR, UNLESS OTHERWISE APPROVED BY THE PRINCIPAL.															
		83. STEEL FLOAT ALL SHOTCRETE SURFACES.																										111. CURING COMPOUNDS SHALL BE SUPPLIED WITH A CERTIFICATE OF COMPLIANCE FROM THE MANUFACTURER IN ACCORDANCE WITH AS 3799 - SECTION 3.3. A COPY OF THIS SHALL BE PROVIDED TO THE PRINCIPAL PRIOR TO CONCRETE BEING MIXED.																134. THE CONTRACTOR SHALL INSTALL TEMPORARY CASING TO PREVENT DRILL-HOLE COLLAPSE WHERE REQUIRED.															
		GABIONS AND RENO MATTRESS																										112. WATER FOR CURING SHALL BE POTABLE WATER WITH A pH BETWEEN 5 AND 7, AND SHALL NOT CONTAIN IMPURITIES IN SUFFICIENT QUANTITY TO CAUSE DISCOLOURATION OF THE CONCRETE.																135. WHERE THE GROUND IS SUSCEPTIBLE TO COLLAPSE CASING SHALL BE USED.															
		84. GABIONS: DOUBLE TWISTED, HEXAGONAL WIRE MESH GABIONS OF NOMINAL 80x100 MESH, WITH 3.4mm O/D FRAME WIRE AND 2.7mm MESH WIRE, COMPLETE WITH DIAPHRAGMS AT 1M CENTRES. ALL COMPONENTS TO BE MECHANICALLY CONNECTED AT THE PRODUCTION FACILITY WITH MINIMUM CONNECTION STRENGTH REQUIREMENTS AS PER TABLE 2 OF ASTM A975, INSTALLED IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS.																										113. STEAM CURING IS NOT PERMITTED.																136. HOLE SHALL BE CLEANED OF ALL LOOSE OR DELETERIOUS MATERIAL ON COMPLETION OF DRILLING AND THE OPENING SEALED TO PREVENT ENTRY FROM FOREIGN MATTER.															
		85. ALL WIRE SHALL BE MILD STEEL, GALMAC COATED (95% ZINC + 5% ALUMINIUM MISCHMETAL ALLOY) TO THE REQUIREMENTS OF ASTM B750-99 AND HEAT BONDED THROUGH EXTRUSION WITH AN ADDITIONAL 0.5MM HEAVY DUTY GREY PVC COATING.																										REINFORCEMENT (FOOTINGS)																SOIL NAIL AND FACING REINFORCEMENT															
		86. GABIONS MUST HAVE A VALID BRITISH BOARD OF AGRÉMENT (BBA) CERTIFICATE FOR GALFAN + PVC WIRE.																										114. STEEL REINFORCEMENT MUST BE IN ACCORDANCE WITH AS 4671, DOWELS MUST COMPLY WITH AS 3679.1.																137. SOIL NAILS TO BE DOUBLE ENCAPSULATED AND HOT-DIP GALVANIZED IN ACCORDANCE WITH AS4680.															
		87. THE GABION FILL MATERIAL SHALL BE IN ACCORDANCE WITH AS 2758.4 - 2000. THE MINIMUM ROCK SIZE SHALL BE 100mm AND THE MAXIMUM ROCK SIZE SHALL BE 250mm.																										115. MESH REINFORCEMENT SIZE MUST BE IN ACCORDANCE WITH DRAWING DETAILS.																138. DRILL HOLES FOR NAILS MUST BE AT LEAST 150mm DIAMETER AND PREPARED AS PER TfNSW R64 U.N.O.															
		88. BIDIM A34 NON-WOVEN GEOTEXTILE (OR APPROVED EQUIVALENT) TO BE PLACED AT ALL ROCKFILL-SOIL INTERFACE, INSTALLED IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS.																										116. REINFORCEMENT MAY BE DISPLACED SLIGHTLY WHERE NECESSARY TO CLEAR DOWELS, ANCHOR BOLTS, FORMED HOLES AND RECESSES.																139. SOIL NAILS TO BE GRADE D500N OR EQUIVALENT HIGH STRENGTH DEFORMED BARS.															
		89. ROCK FILL MATERIAL BEHIND THE GABION SHALL HAVE A MINIMUM SIZE OF 75mm AND MAXIMUM SIZE OF 200mm. ROCK FILL TYPE A MATERIAL SHALL BE IN ACCORDANCE WITH AS 2758.4-2000.																										117. AUSTRALIAN STANDARD BAR SHAPES ARE IN ACCORDANCE WITH AS 1100.501.																140. SOIL NAILS MUST BE INSTALLED AS SHOWN IN THE SCHEDULES.															
		PILES AND FOUNDATIONS																										118. BAR SIZE IS THE NOMINAL DIAMETER IN MILLIMETRES, OR THE AS/NZS 4671 FABRIC NUMBER.																141. PROVIDE CENTRALISERS FOR NAILS AT INTERVAL NOT EXCEEDING 2000mm.															
		90. THE BEARING CAPACITY AT FOUNDING LEVEL SHALL BE VERIFIED BY A SUITABLY QUALIFIED GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF CONCRETE.																										119. THE GRADE OF REINFORCEMENT, IF NOT STATED ON THE DRAWINGS SHALL BE D500N TO AS/NZS 4671.																142. STEEL MESH MUST BE GRADE D500N IN ACCORDANCE WITH AS4671.															
		91. THE FOUNDATION MATERIAL SHALL BE INSPECTED BY A SUITABLY QUALIFIED GEOTECHNICAL ENGINEER TO CONFIRM THE EXPOSED FOUNDATION MATERIAL SATISFIES THE DESIGN ASSUMPTIONS. ANY MATERIAL ASSESSED TO BE UNSUITABLE (INCLUDING EXISTING FILL) SHALL BE REMOVED AND REPLACED.																										120. BAR BENDING AND HOOK DETAILS SHALL BE IN ACCORDANCE WITH SECTION 5.13 OF AS 5100.																143. BEARING PLATE MUST BE GRADE D250N IN ACCORDANCE WITH AS3678															
		92. CLEANLINESS OF PILE BASES SHALL BE CONFIRMED ON SITE BY A SUITABLY QUALIFIED GEOTECHNICAL ENGINEER PRIOR TO CASTING OF PILES.																										121. LAPS NOT SHOWN ON THE DRAWINGS SHALL BE STAGGERED SO THAT NO MORE THAN 50% OF BARS ARE LAPPED IN ANY CROSS SECTION.																144. SOIL NAIL FACING SHOTCRETE COVER TO REINFORCEMENT FROM AIR = 50mm															
		93. ALL BORED PILE WORK SHALL BE IN ACCORDANCE WITH AS2159.																										122. REINFORCEMENT SYMBOLS COMMONLY USED ARE: N DENOTES GRADE 500 MPa N BARS TO AS 4671. R DENOTES GRADE 250 R HOT ROLLED PLAIN BARS TO AS 4671. SL DENOTES SQUARE REINFORCING FABRIC TO AS 4671. RL DENOTES RECTANGULAR REINFORCING FABRIC TO AS 4671. LTM DENOTES TRENCH MESH WHICH SHALL COMPLY WITH AS 4671.																145. SOIL NAIL FACING SHOTCRETE COVER TO REINFORCEMENT FROM GROUND = 75mm															
		94. STEEL CASING (IF REQUIRED) AND REINFORCING CAGE SHALL BE SECURELY AND ACCURATELY HELD IN POSITION DURING CONCRETE PLACEMENT.																										123. THE METHOD USED TO LABLE REINFORCEMENT ON THE DRAWINGS IS AS FOLLOWS:																146. SOIL NAIL 40mm COVER IS ACCEPTABLE TO STRIP DRAIN. CONSTRUCTION METHODOLOGY TO ENSURE COVER TO STRIP DRAIN IS ACHIEVED. LOCAL ADDITIONAL EXCAVATION MAY BE REQUIRED.															
		95. BORED PILES SHALL BE CONCRETED ON THE DAY OF APPROVAL UNLESS PERMISSION IS GIVEN OTHERWISE.																																										147. STEEL MESH MUST BE LAPPED AT ONE LOCATION NEATLY BY THREE WIRES SO AS NOT TO CREATE SHADOW IN THE SHOTCRETE.															
		96. BORED PILE TOE LEVELS, WHERE SHOWN ARE ESTIMATES ONLY AND SHALL BE ESTABLISHED DURING SITE INSPECTION OF WORK IN PROGRESS.																																										148. ALL STEEL MESH LAPS AND STARTER BARS SHALL BE CLEANED TO BARE MESH PRIOR TO FIXING THE ADJOINING STEEL OR SPRAYING THE NEXT PANEL.															
		CONCRETE																																										149. STEEL MESH LAPS MUST BE STAGGERED AND CUT SUCH THAT ONLY TWO SHEETS ARE LAPPED AT ONE LOCATION. LAPS ARE NOT ALLOWED OVER STRIP DRAINS.															
		97. MINIMUM 28 DAY CHARACTERISTIC COMPRESSIVE STRENGTH OF ALL CONCRETE IS 40 MPa.																																										150. NAILS, BEARING PLATES, WASHERS AND ALL STEEL REINFORCEMENT MUST BE HOT DIP GALVANISED AND IN ACCORDANCE WITH AS4680 WITH A COATING WEIGHT OF 600G/M2.															
		98. COVER TO REINFORCEMENT NEAREST TO THE CONCRETE SURFACE SHALL BE 50MM UNLESS NOTED OTHERWISE.																																										151. NAIL NUTS MUST BE GRADE C COMPLYING WITH AS1112.3 AND PROPERTY CLASS 5 COMPLYING WITH AS4291.2 OR EQUIVALENT TO SUIT THE THREADED END OF SOIL NAIL.															
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		THIS DRAWING IS THE PROPERTY OF SLR CONSULTING AUSTRALIA AND MUST NOT BE RETAINED, COPIED OR USED WITHOUT THE CONSENT OF THE COMPANY.						THIS DRAWING IS NOT TO BE USED FOR CONSTRUCTION UNLESS ENDORSED BELOW						Responsible Principal Signature		Date		DRAWN: WP/GL		DATE: 20.10.2022		DESIGN: MB/DT		DATE: 19.10.2022		DRG. CHECK: MB		DATE: 24.10.2022		DES. CHECK: DT		DATE: 25.10.2022		 <div>LEVEL 1, THE CENTRAL BUILDING INNOVATION CAMPUS, SQUIRES WAY NORTH WOLLONGONG NSW 2500 AUSTRALIA T: +61 2 4249 1004 F: +61 2 4249 1000 www.slrconsulting.com</div>  <div>The content contained within this document may be based on third party data. SLR Consulting Australia Pty Ltd does not guarantee the accuracy of any such information.</div>		FOR CONSTRUCTION		A1		DO NOT SCALE THIS DRAWING IF IN DOUBT ASK		SCALE: N/A		DRAWING NUMBER: 660.30255-G-1003				ISSUE: 1											

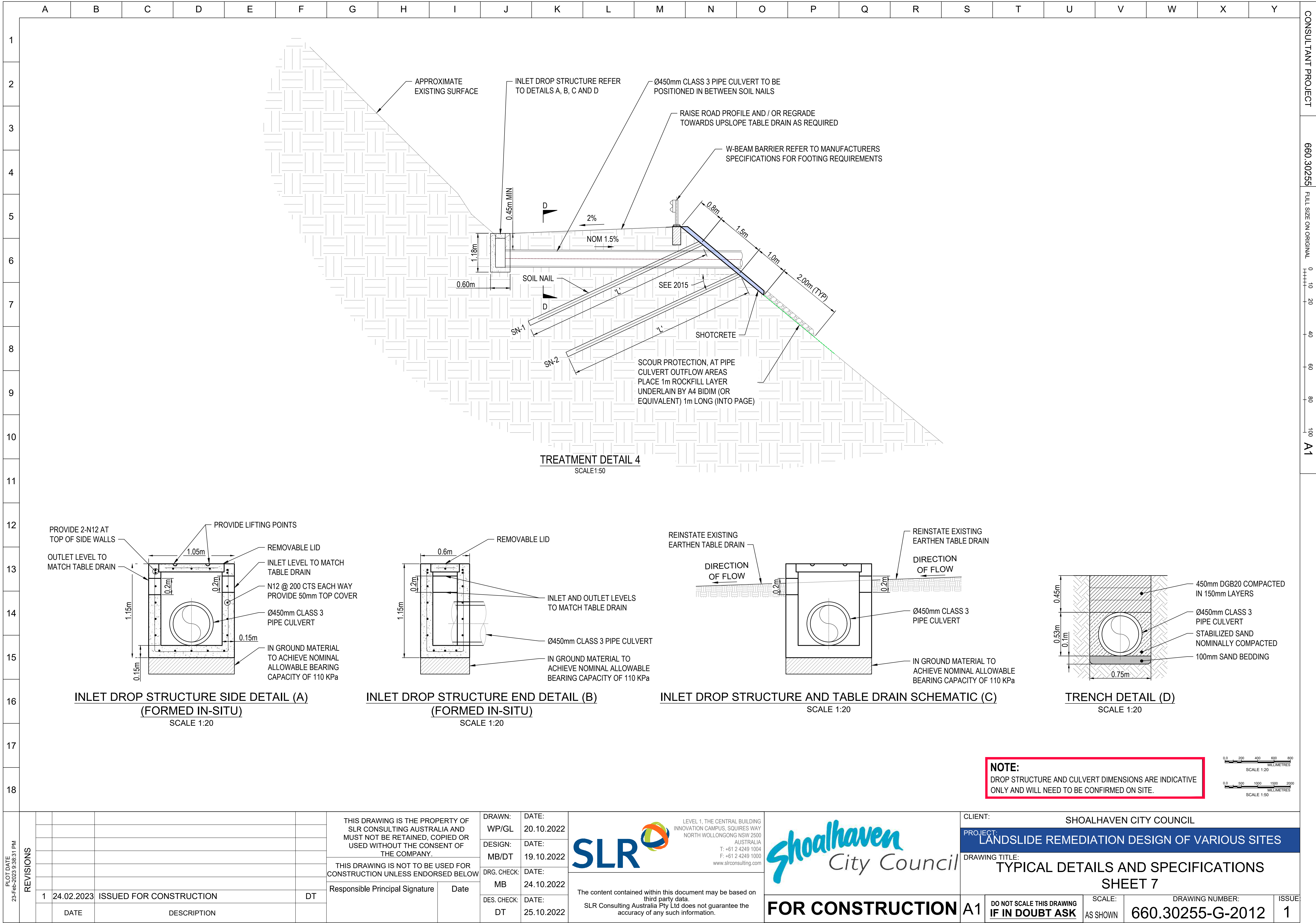
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1	2	GROUT REQUIREMENTS																							CONSULTANT PROJECT		
		152. GROUTS MUST HAVE HIGH BLEED RESISTANCE, LOW SHRINKAGE AND HIGH FLUIDITY. DO NOT USE ADDITIVES OR ADMIXTURES WITHOUT THE APPROVAL OF THE PRINCIPAL.																									
3	4	153. MINIMUM COMPRESSIVE STRENGTH OF GROUT MUST BE 40MPA AT 28 DAYS.																							660.30255		
		154. GROUT FLUIDITY, BLEED AND COMPRESSIVE STRENGTH TESTING TO BE IN ACCORDANCE WITH TFNSW R64 CLAUSE 3.4.3																									
5	6	SOIL NAIL TESTING PROCEDURE																							FULL SIZE ON ORIGINAL		
		155. NAIL TESTING TO BE CONDUCTED IN ACCORDANCE WITH TFNSW R64.																									
7	8	156. CONTRACTOR TO CONFIGURE TEST NAILS (INCLUDING NAIL DIAMETER AND BEARING PLATE) TO ENSURE 200% OF THE WORKING BOND STRESS CAN BE ACHIEVED PRIOR TO 80% OF THE BAR YIELD LOAD.																							0		
		157. SUITABILITY TESTING ARE TO BE COMPLETED PRIOR TO INSTALLATION OF PRODUCTION NAILS.																								10	
9	10	158. A TOTAL OF 3% OF PERMANENT NAILS SHALL BE SUBJECTED TO ACCEPTANCE TESTS .																							20		
		159. LOCATIONS OF THE SUITABILITY AND ACCEPTANCE TEST SOIL NAILS TO BE CONFIRMED ON SITE.																								40	
11	12	SHOTCRETE																							60		
		160. SHOTCRETE TO BE IN ACCORDANCE WITH TFNSW R68.																								80	
13	14	161. MINIMUM COMPRESSIVE STRENGTH OF SHOTCRETE MUST BE 40MPA AT 28 DAYS. USE SHRINKAGE LIMITED CEMENT IN SHOTCRETE IN ACCORDANCE WITH SPECIFICATION TFNSW R68 AND TFNSW QA3211.																							100		
		162. SHOTCRETE MIX DESIGN TO EXPOSURE CLASSIFICATION B:2 IN ACCORDANCE WITH TFNSW R64 CLAUSE 2.3.3.																								A1	
15	16	163. SHOTCRETE QUALITY TESTING IN ACCORDANCE WITH TFNSW R68 CLAUSE 8.5 AND ANNEXURE R68/L.																									
		164. APPROPRIATE THICKNESS GAUGES SHALL BE FIXED TO THE STEELWORKS TO ALLOW PROOF OF ADEQUATE COVER.																									
17	18	FIBRECRETE																									
		165. FIBRECRETE TO BE IN ACCORDANCE WITH TFNSW B82.																									
19	20	166. MINIMUM COMPRESSIVE STRENGTH OF FIBRECRETE MUST BE 40MPA AT 28 DAYS. USE SHRINKAGE LIMITED CEMENT IN FIBRECRETE IN ACCORDANCE WITH SPECIFICATION TFNSW B82 AND TFNSW QA3211.																									
		167. FIBRECRETE MIX DESIGN TO EXPOSURE CLASSIFICATION B:2 IN ACCORDANCE WITH TFNSW R64 CLAUSE 2.3.3.																									
21	22	168. SHOTCRETE QUALITY TESTING IN ACCORDANCE WITH TFNSW B82.																									
		169. APPROPRIATE THICKNESS GAUGES SHALL BE FIXED TO THE STEELWORKS TO ALLOW PROOF OF ADEQUATE COVER.																									
23	24	DRAINAGE (SOIL NAILING)																									
		170. 150mm HDPE STRIP FILTER DRAIN (FLEXIBLE CUSPATED DRAINS) TO BE IN ACCORDANCE WITH TFNSW QA3557.																									
25	26	171. BOTTOM OF THE STRIP DRAINS DAYLIGHT AT BOTTOM OF SHOTCRETE WALL FACING.																									
		172. HDPE STRIP DRAIN TO BE CORRUGATED CORE PERFORATED AND MUST BE INSTALLED DIAGONALLY ACROSS SLOPE FACE AT 45°.																									
27	28	173. STRIP DRAINS MUST BE CHASED INTO SOIL SLOPE TO ALLOW FULL SHOTCRETE THICKNESS AT ALL LOCATIONS.																									
		174. WHERE CONNECTION IS TO BE MADE TO AN EXISTING DRAINAGE STRUCTURE OR OPEN DRAIN THE POSITION AND LEVEL OF EXISTING DRAINAGE STRUCTURE SHALL BE CONFIRMED PRIOR TO CONSTRUCTION. BLEND CONNECTIONS SMOOTHLY TO ENSURE PROPER DRAINAGE AT ALL POINTS.																									
29	30	SPECIFICATIONS (SOIL NAILING)																									
		175. ALL SOIL NAILING MATERIALS AND WORK IS TO COMPLY WITH THE FOLLOWING TFNSW PROJECT SPECIFIC SPECIFICATIONS:																									
31	32	A. R64 SOIL NAILING																									
		B. R68 SHOTCRETE WITHOUT FIBRES																									
33	34	C. B82 SHOTCRETE WITH FIBRES																									
		D. R178 REVEGETATION																									
35	36	176. ALL SOIL NAILING MATERIALS AND WORK IS TO COMPLY WITH THE FOLLOWING TFNSW QA STANDARD SPECIFICATIONS:																									
		A. R23 PLASTIC FLEXIBLE PIPES																									
37	38	B. R53 CONCRETE FOR GENERAL USE MORTAR AND GROUT																									
		C. R55 ROCK FILLED GABIONS AND MATTRESSES																									
39	40	D. R63 GEOTEXTILES																									
		E. R71 UNBOUND AND MODIFIED PAVEMENT COURSE																									
41	42	F. 3557 FLEXIBLE STRIP FILTER DRAINS																									
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							PROJECT:			LANDSLIDE REMEDIATION DESIGN OF VARIOUS SITES					
							DRAWING TITLE:			SITE DM00899 GENERAL ARRANGEMENT LAYOUT PLAN					
		1	24.02.2023	ISSUED FOR CONSTRUCTION	DT	THIS DRAWING IS NOT TO BE USED FOR CONSTRUCTION UNLESS ENDORSED BELOW				DRG. CHECK:	DATE:	The content contained within this document may be based on third party data. SLR Consulting Australia Pty Ltd does not guarantee the accuracy of any such information.			
						Responsible Principal Signature	Date			MB	24.10.2022				
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